

GENERAL

THESE DRAWINGS SHALL READ IN CONJUNCTION WITH THE ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AS WELL AS WITH OTHER WRITTEN INSTRUCTIONS THAT MAY BE ISSUED DURING THE CONSTRUCTION. ANY DISCREPANCIES SHALL BE FORWARDED TO THE ENGINEER FOR CLARIFICATION PRIOR TO COMMENCING THE WORK.

ALL DIMENSIONS ARE IN MILLIMETERS UNLESS NOTED OTHERWISE (UNO).

ALL DIMENSIONS, LEVELS AND SETTING OUT INFORMATION THAT ARE SHOWN ON THESE DRAWINGS SHALL BE CHECKED ON SITE PRIOR TO COMMENCING CONSTRUCTION WORK. ANY DISCREPANCIES SHALL BE REFERRED TO THE ENGINEER.

DIMENSIONS SHALL NOT BE OBTAINED BY SCALING THESE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL DRAWINGS FOR OTHER DIMENSIONS.

ALL MATERIALS AND CONSTRUCTION WORK SHALL BE IN ACCORDANCE WITH THE RELEVANT S.A.A CODES AND OTHER STATUTORY REQUIREMENTS OF THE RELEVANT BUILDING AUTHORITY.

THE STRUCTURE SHALL BE MAINTAINED IN A STABLE CONDITION AND NO PART OF THE STRUCTURE SHALL BE OVERSTRESSED DURING CONSTRUCTION.

THE STABILITY OF EXISTING STRUCTURES AND FOUNDATIONS SHALL BE ENSURED DURING CONSTRUCTION.

DESIGN LIVE LOADS ARE AS PER AS1170.1 AND THE FOLLOWING:

AREA	UNIFORM	POINT
FLOORS - INTERNAL	1.5 kPa	1.8 kN
FLOORS - EXTERNAL	2 kPa	1.8 kN
GARAGE AND DRIVEWAY	2.5 kPa - 5 kPa	13 kN - 31 kN
ROOF AREAS	0.5 kPa	1.4 kN

WIND LOADS ARE IN ACCORDANCE WITH AS1170.2 AND AS4055

EARTHQUAKE LOADINGS ARE IN ACCORDANCE WITH AS1170.4

SNOW LOADINGS ARE IN ACCORDANCE WITH AS1170.3.

DESIGN WIND CATEGORY - N3

DESIGN SNOW LOAD - 22.308 kPa

(PERISHER VALLEY ALTITUDE = 1740m)

FOOTINGS AND FOUNDATIONS

ALL EARTHWORKS SHALL BE CARRIED OUT IN ACCORDANCE WITH THE GEOTECHNICAL REPORT, OR NOTES PROVIDED ON THESE DRAWINGS.

THE SITE HAS BEEN CLASSIFIED AS CLASS 'M' IN ACCORDANCE WITH AS2870.

FOOTINGS HAVE BEEN DESIGNED FOR AN ALLOWABLE BEARING PRESSURE OF 200 kPa IN STIFF CLAY.

FOOTINGS SHALL BEAR ON UNDISTURBED NATURAL GROUND, CLEAR OF ORGANIC MATERIAL.

FOOTINGS SHALL BE DEWATERED AND CLEANED PRIOR TO POURING CONCRETE.

FOOTINGS SHALL BE INSPECTED AND APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO CONCRETE POUR.

FOOTINGS TO BE BACKFILLED AS SOON AS POSSIBLE TO AVOID A REDUCTION IN THE BEARING CAPACITY OF THE FOUNDATION MATERIAL DUE TO SOFTENING BY WATER OR DRYING OUT.

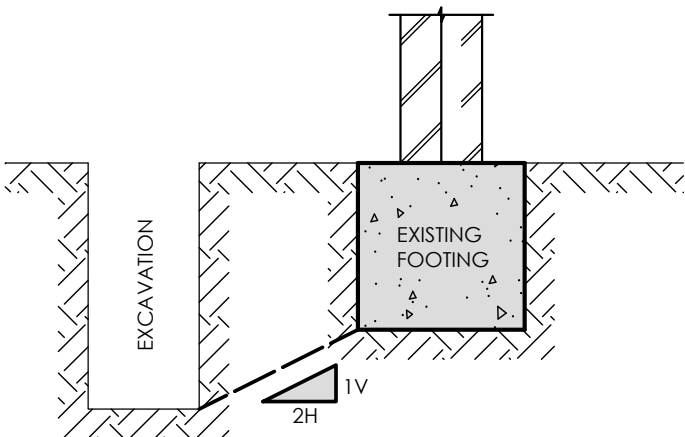
IF VARIABLE BEARING STRATA IS ENCOUNTERED DURING EXCAVATION FOR FOOTINGS, EXCAVATION SHALL CONTINUE TILL A UNIFORM MATERIAL IS ENCOUNTERED OF EQUAL OR GREATER STRENGTH THAN SPECIFIED.

FOOTINGS SHALL BE LOCATED CENTRALLY UNDER WALLS AND COLUMNS UNO.

COMPACTED FILL SHALL ONLY BE USED WITH THE APPROVAL OF THE GEOTECHNICAL ENGINEER. ALL ORGANIC MATERIAL, TOPSOIL AND UNCONTROLLED FILL TO BE STRIPPED TO MINIMUM DEPTH OF 150MM. FILL SHALL BE IN 200MM LAYERS USING AN APPROVED GRANULAR MATERIAL COMPACTED TO 98% STANDARD MAXIMUM DRY DENSITY.

FILL MATERIAL UNDER SLABS SHALL BE IN ACCORDANCE WITH AS2870.

EXCAVATIONS NEAR NEW OR EXISTING FOOTINGS SHALL NOT BE WITHIN THE INFLUENCE LINE OF THE FOOTING. UNLESS OTHERWISE APPROVED, THIS INFLUENCE LINE SHALL BE TAKEN AS 1 VERTICAL TO 2 HORIZONTAL AS SHOWN BELOW:



EXCAVATION ZONE OF INFLUENCE

GZCONSULTINGENGINEERS PTY LTD

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PROPOSED PORTICO AT 29 WHEATLEY ROAD, PERISHER VALLEY, NSW 2624 CHALET SONNENHOF

REINFORCED CONCRETE

ALL WORKMANSHIP AND MATERIALS TO BE IN ACCORDANCE WITH AS3600.

UNLESS NOTED OTHERWISE CONCRETE IS TO HAVE THE FOLLOWING QUALITIES
MAX AGGREGATE SIZE - 20mm
SLUMP - 80mm

CONCRETE STRENGTH GUIDE (F _c)	
SLABS ON GROUND	32 MPa U.N.O
PIERS AND FOOTINGS	32 MPa U.N.O
SUSPENDED SLABS	40 MPa U.N.O
COLUMNS	40 MPa U.N.O
BLOCK CORE FILLING	32 MPa (10mm AGGREGATE)

NOTE: ANY HIGHER CONCRETE STRENGTH SHOWN ON A SPECIFIC DRAWING SHALL BE ADOPTED IN LIEU OF THIS TABLE.

ANY ADMIXTURES USED IN THE CONCRETE ARE TO BE APPROVED BY THE ENGINEER.

CONCRETE COVERS MUST BE MAINTAINED THROUGH ALL CHAMFERS, DRIP GROOVES, PENETRATIONS, ETC. REFER TO DRAWING FOR CONCRETE COVERS.

ALL CONCRETE SHALL BE MECHANICALLY VIBRATED. VIBRATORS SHALL NOT BE USED TO SPREAD CONCRETE.

ALL CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ENGINEER. NO HOLES OR CHASES OTHER THAN THOSE SHOWN ON THE DRAWINGS TO BE MADE IN THE CONCRETE WITHOUT PRIOR APPROVAL OF THE ENGINEER.

SIZES OF CONCRETE ELEMENTS SHOWN ON THE DRAWINGS DO NOT INCLUDE THE THICKNESS OF APPLIED FINISHES.

WHERE SLABS OR BEAMS ARE CAST AGAINST NON LOAD BEARING MASONRY WALLS, A 10mm STYRENE SEPARATION MATERIAL SHALL BE PROVIDED AT THE INTERFACE.

CONDUITS AND PIPES SHALL BE PLACED IN THE MIDDLE THIRD OF THE CONCRETE AND SPACED AT NO LESS THAN 3 DIAMETERS. REINFORCEMENT SHALL BE GRADE D500N UNLESS NOTED OTHERWISE.

REINFORCEMENT SYMBOLS ARE AS FOLLOWS

N - GRADE 500N NORMAL DUCTILITY DEFORMED BAR
R - GRADE 250N NORMAL DUCTILITY ROUND BAR
SL - GRADE 500L LOW DUCTILITY WELDED DEFORMED SQUARE MESH
RL - GRADE 500L LOW DUCTILITY WELDED DEFORMED RECTANGLE MESH
LTM - GRADE 500L LOW DUCTILITY WELDED DEFORMED TRENCH MESH.
S - GRADE 250N NORMAL DUCTILITY DEFORMED BAR (FOR POOLS)

REINFORCEMENT ON THE DRAWINGS IS SHOWN DIAGRAMMATICALLY AND DOES NOT NECESSARILY SHOW ITS TRUE PROJECTION.

WELDING OF REINFORCEMENT SHALL NOT BE PERMITTED UNLESS SHOWN ON THESE DRAWINGS OR APPROVED BY THE ENGINEER.

SPLICES AND COGS IN REINFORCEMENT SHALL BE IN ACCORDANCE WITH AS3600 AND NO LESS THAN THE FOLLOWING:

BAR	SPLICE LENGTH	COG LENGTH
N12	400	200
N16	600	225
N20	800	275
N24	1000	325
N28	1200	375

REINFORCED CONCRETE CONTINUED

MESH SHALL BE LAPPED TWO TRANSVERSE WIRES (ONE COMPLETE SQUARE) PLUS 25mm.

ALL REINFORCEMENT IS TO FIRMLY SUPPORTED IN ITS POSITION BY THE USE OF BAR CHAIRS SPACED AT NO GREATER THAN 750mm. THIS APPLIES TO BOTH BARS AND MESH.

REINFORCEMENT IS TO BE HELD IN PLACE VIA THE USE OF BARS TIES AT A MAXIMUM SPACING OF EVERY ALTERNATE BAR INTERSECTION.

ALL FORMWORK MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH AS3610 AND AS3600.

FORMWORK SURFACES SHALL BE THOROUGHLY CLEANED AND PRE WETTED PRIOR TO CONCRETE BEING POURED.

APPROVED SPRAY ON CURING COMPOUNDS MAY BE USED PROVIDED THEY DO NOT INTERFERE WITH ANY PROPOSED FLOOR FINISHES.

STRIPPING TIMES FOR REINFORCEMENT SHALL BE IN ACCORDANCE WITH AS3610 OR AS DIRECTED BY THE ENGINEER.

CURING OF CONCRETE SHALL BE ACHIEVED BY KEEPING EXPOSED SURFACES CONTINUOUSLY WET FOR PERIOD OF 3 DAYS, FOLLOWED BY A PREVENTION OF LOSS OF MOISTURE FOR SEVEN DAYS, FOLLOWED BY A GRADUAL DRYING OUT PERIOD.

CHEMICALLY ANCHORED REINFORCEMENT

WHERE SHOWN ON THE DRAWINGS REINFORCEMENT BARS SHALL BE CHEMICALLY ANCHORED INTO EXISTING CONCRETE AS DESCRIBED BELOW.

PERCUSSION DRILL (CORING NOT PERMITTED) A HOLE TO THE CORRECT DIAMETER AND DEPTH FOR THE PARTICULAR SIZE REINFORCING BAR AS TABULATED BELOW, UNLESS SHOWN OTHERWISE ON THE DRAWINGS.

BAR SIZE	HOLE DIA (mm)	HOLE DEPTH (mm)
12	16	125
16	22	170
20	28	205

THOROUGHLY CLEAN THE HOLE USING A ROUND WIRE BRUSH AND BLOW OUT ALL DUST.

ENSURE HOLE IS CLEAN AND DRY AND INSERT SUFFICIENT HILTI HY 150 RESIN INTO THE BASE OF THE HOLE TO ENSURE THAT WHEN THE BAR IS INSTALLED RESIN APPEARS AT THE FACE OF THE HOLE.

IMMEDIATELY INSERT THE REINFORCING BAR INTO THE HOLE BY ROTATING SLOWLY TO FULLY COAT THE BAR WITH RESIN, AND PUSH FULLY INTO THE HOLE.

ENSURE BAR IS NOT DISTURBED WHILST RESIN IS CURING. (APPROX. 2 HOURS).

PERMANENT METAL FORMWORK

PERMANENT METAL FORMWORK SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.

UNLESS NOTED OTHERWISE PERMANENT METAL FORMWORK SHALL BE 1.00mm BTM.

THE PERMANENT METAL FORMWORK SHALL BE SUITABLY PROPPED AND HELD DOWN TO PREVENT DISPLACEMENT DURING CONCRETE PLACEMENT.

PROPPING SHALL NOT BE REMOVED TILL THE CONCRETE HAS REACHED SUFFICIENT STRENGTH.

EACH SHEET SHALL BE FIXED TO THE SUPPORTING STRUCTURE VIA SPOT WELDING OR FASTENERS.

THE PERMANENT METAL FORMWORK SHALL NOT BE SPLICED OR JOINED AT MIDSPAN. A MINIMUM END BEARING OF 50mm SHALL BE PROVIDED.

DRAWING SHEET SCHEDULE

N1	COVER SHEET & STRUCTURAL NOTES 1
N2	STRUCTURAL NOTES 2
S1	RAFT SLAB & ROOF FRAMING PLAN
S2	STRUCTURAL SECTION & DETAILS



Department of Planning
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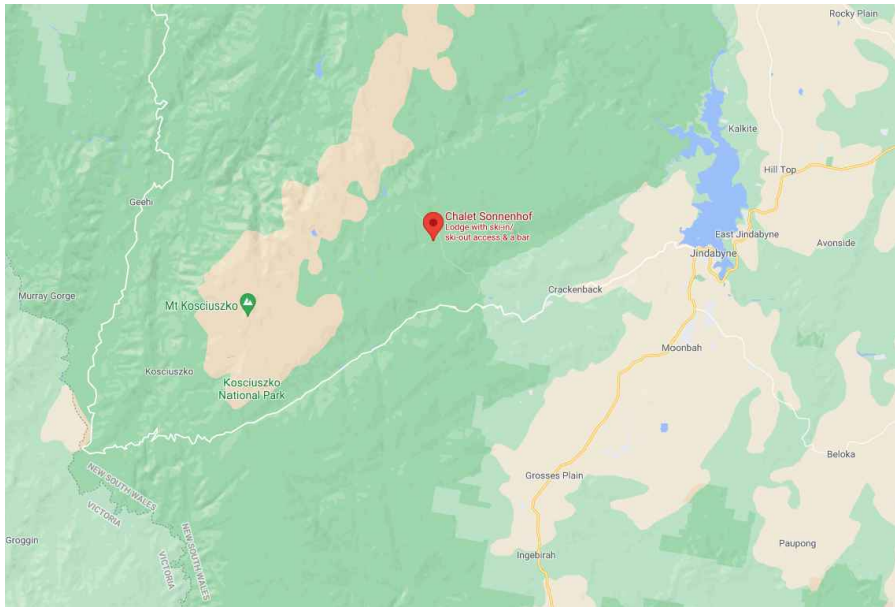
Issued under the Environmental Planning and Assessment Act 1979

Approved Application No DA 22/11518

Granted on the 16 December 2022

Signed E Liang

Sheet No 11 of 20



LOCATION MAP (N.T.S)



INSPECTIONS

IT IS ADVISED THAT GZ CONSULTING ENGINEERS INSPECT ALL STAGES OF THE STRUCTURAL WORKS. INSPECTIONS WHICH WOULD BE REQUIRED TO CERTIFY THE WORKS ARE:
• SLAB ON GROUND
• ROOF FRAMING

ALL INSPECTIONS MUST BE CLEARLY VISIBLE, BEFORE VAPOUR BARRIER, CONCRETE OR LINING PLACEMENT.

PLEASE NOTE
ANY ELEMENTS NOT INSPECTED BY THE ENGINEER CANNOT BE CERTIFIED BY THE ENGINEER. THE ENGINEER IS TO BE GIVEN 24 HOURS MINIMUM NOTICE FOR INSPECTIONS.

STATUS				
PRELIMINARY				
SHEETS IN SET	DATE	DRAWN	DESIGN	SIGNED
04	10.09.21	J.J	G.Z	
PROJECT No.		DRAWING No.		SHEET SIZE
SONNENHOFF		N1		A2

MASONRY
ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH AS3700.

ALL LOAD BEARING WALLS SHALL HAVE A SLIP JOINT WHEN SUPPORTING CONCRETE SLABS OR BEAMS. TWO LAYERS OF APPROVED GREASED GALVANIZED STEEL SHALL BE PLACED ON SMOOTH BRICKWORK OR TOWELED MORTAR FINISH.

NON LOAD BEARING MASONRY WALLS SHALL BE SEPARATED FROM CONCRETE SLABS AND BEAMS BY A 10mm STYRENE SEPARATION MATERIAL AT THEIR INTERFACE.

NO MASONRY WALLS ARE TO BE CONSTRUCTED ON SUSPENDED SLABS UNTIL ALL PROPPING HAS BEEN REMOVED AND THE CONCRETE HAS REACHED ITS SPECIFIED 28 DAY STRENGTH.

VERTICAL CONTROL JOINTS TO BE SPACED AT A MAXIMUM OF 8M CENTERS AND 4M MAXIMUM FROM CORNERS AND AT THE INTERFACE BETWEEN NEW AND EXISTING MASONRY, IN ACCORDANCE WITH AS3700 UNLESS NOTED OTHERWISE OR APPROVED BY THE ENGINEER.

COMPRESSIVE STRENGTH OF MASONRY SHALL BE MINIMUM 20 MPA.

UNLESS NOTED OTHERWISE MORTAR FOR MASONRY SHALL BE CEMENT:LIME:SAND IN THE RATIO OF 1:1:6.

PROVIDE WALL TIES AT A 600MM CENTERS MAXIMUM BOTH VERTICALLY AND HORIZONTALLY. WALL TIES SHALL BE STAINLESS STEEL BELOW DAMP PROOF COURSE AND GALVANIZED ABOVE.

NO CHASES OR RECESSES ARE PERMITTED IN LOAD BEARING MASONRY UNLESS APPROVED BY THE ENGINEER.

CAVITIES ARE TO BE KEPT CLEAN AND CLEAR AT ALL TIMES. SIZE OF CAVITIES SHALL BE NOT LESS THAN 40MM AND NOT EXCEED 200MM.

BLOCKWORK
ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH AS3700.

CHARACTERISTIC COMPRESSIVE STRENGTH OF BLOCKWORK SHALL BE 15MPa.

BLOCKWORK SHALL BE DOUBLE 'U' BLOCK CONSTRUCTION.

CORE FILL CONCRETE IS TO HAVE A COMPRESSIVE STRENGTH OF 20MPa WITH 10mm AGGREGATE AND A SLUMP OF 230mm.

MAXIMUM POUR HEIGHT FOR UNPROPPED CORE FILLED WALLS SHALL BE 2M.

55mm MINIMUM COVER TO REINFORCEMENT SHALL BE PROVIDED FROM THE OUTSIDE FACE OF THE WALL.

REINFORCEMENT SPLICE LENGTHS ARE TO BE IN ACCORDANCE WITH AS3600 AND CONCRETE NOTES SPLICES & COG TABLE.

CLEAN OUT HOLES ARE TO BE PROVIDED AT THE BASE OF ALL CORE FILLED MASONRY WALLS.

NO MASONRY WALLS ARE TO BE CONSTRUCTED ON SUSPENDED SLABS UNTIL ALL PROPPING HAS BEEN REMOVED AND THE CONCRETE HAS REACHED ITS SPECIFIED 28 DAY STRENGTH.

VERTICAL CONTROL JOINTS TO BE SPACED AT A MAXIMUM OF 8m CENTERS AND 4m MAXIMUM FROM CORNERS AND AT THE INTERFACE BETWEEN NEW AND EXISTING MASONRY, IN ACCORDANCE WITH AS3700 UNLESS NOTED OTHERWISE OR APPROVED BY THE ENGINEER.

VERTICAL CONTROL JOINTS SHALL BE REINFORCED WITH N20-400 DOWELS 600mm LONG WITH ONE END GREASED AND SHEATHED.

UNLESS NOTED OTHERWISE MORTAR FOR MASONRY SHALL BE CEMENT:LIME:SAND IN THE RATIO OF 1:1:6.

NO CHASES OR RECESSES ARE PERMITTED IN LOAD BEARING MASONRY UNLESS APPROVED BY THE ENGINEER.

NO ADMIXTURES ARE TO BE USED IN THE MORTAR OR CORE FILL CONCRETE WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER.

LINTEL BEARING
OPENING LESS THAN 1m = 100mm MIN. BEARING EACH END. OPENING GREATER THAN 1m = 150mm MIN. BEARING EACH END.

RETAINING WALL AG. DRAIN
PROVIDE 100mm DIAMETER AGRICULTURE DRAIN WRAPPED IN GEOFABRIC GRADED TO OUTLET.

STRUCTURAL STEEL
ALL DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEELWORK TO BE IN ACCORDANCE WITH AS4100.

UNLESS NOTED OTHERWISE, THE FOLLOWING SHALL APPLY FOR STEEL SECTIONS AND PLATES:

HOT ROLLED UB, UC, PFC & EA SECTIONS TO BE GRADE 300PLUS TO AS/NZS 3679 SQUARE, RECTANGULAR & CIRCULAR HOLLOW SECTIONS TO BE GRADE 350 TO AS 1163 STEEL PLATES SHALL BE GRADE 250 TO AS/NZS 3678.

COLD FORMED CEE & ZED PURLINS TO BE GRADE 550/500/450 TO AS/NZS 4600.

BOLT DESIGNATION
4.6/S COMMERCIAL BOLTS TO AS 1111, SNUG TIGHTENED
8.8/S HIGH STRENGTH STRUCTURAL BOLTS TO AS 1511, SNUG TIGHTENED
8.8/TB HIGH STRENGTH STRUCTURAL BOLTS TO AS 1511, FULLY TENSIONED

BEARING JOINT
8.8/TF HIGH STRENGTH STRUCTURAL BOLTS TO AS 1511, FULLY TENSIONED.

FRICTION JOINT
(ALL BOLTS SHALL BE FITTED WITH APPROPRIATE WASHERS IN ACCORDANCE WITH AS4100. LOAD INDICATING WASHERS SHALL BE USED FOR TENSIONED CONNECTIONS TF/TB).

ALL BOLTS SHALL BE GALVANISED GRADE 8.8/S UNO. UNLESS SHOWN OTHERWISE ON THE DRAWINGS, ALL CONNECTIONS SHALL CONSIST OF 2M16 8.8/S BOLTS, WITH 10mm PLATES AND 6MM CONTINUOUS FILLETS WELDS.

BOLTS FOR PURLINS SHALL BE 2M12 4.6/S.

CARE SHALL BE TAKEN DURING POURING OF CONCRETE TO ENSURE CAST-IN HOLDING DOWN BOLTS ARE MAINTAINED IN THEIR CORRECT POSITION.

ALL WELDING SHALL BE IN ACCORDANCE WITH AS/NZS 1554 UNO. ALL WELDS SHALL BE 6MM CONTINUOUS FILLET WELDS TYPE SP USING E41XX ELECTRODES. ALL BUT WELDS SHALL BE COMPLETE PENETRATION BUTT WELDS, CATEGORY SP.

GROUTING UNDER BASE PLATES AND ANCHOR BOLTS SHALL CONSIST OF 25mm HIGH STRENGTH, NON SHRINK GROUT.

UNO PROTECTIVE COATING TO ALL STRUCTURAL STEEL SHALL BE AS FOLLOWS:

INTERNAL
WIRE BRUSH CLEAN THEN APPLY 2 COATS ALKYD PRIMER WITH A TOTAL DRY FILM THICKNESS OF 70 MICRONS.

EXTERNAL
BLAST CLEAN SURFACE, THEN APPLY ONE OF THE FOLLOWING COATING SYSTEMS

- HOT DIP GALVANISE 300 G/M² MINIMUM
- HOT DIP GALVANISE 100 G/M² MINIMUM, PLUS 2 COATS VINYL GLOSS OR ALKYD PRIMER.

ANY DAMAGE CAUSED DURING TRANSPORT, ERECTION AND OTHER SITE WORKS, SHALL BE REPAIRED AS PER ORIGINAL SPECIFICATION UNLESS OTHER WRITTEN INSTRUCTIONS ARE GIVEN BY THE ENGINEER.

GALVANISED COATINGS SHALL BE REPAIRED USING ZINC RICH EPOXY PRIMER (3 COATS MINIMUM). AREA TO BE PREPARED AND CLEANED VIA WIRE BRUSH.

WORKSHOP DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF FABRICATION UNO.

TIMBER NOTES
ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH AS1684 AND AS1720. SOFTWOOD TO BE A MINIMUM OF F7 AND HARDWOOD TO BE MINIMUM OF F14 UNLESS NOTED OTHERWISE.

SOFTWOOD TIMBER FRAMING TO HAVE A MINIMUM PROTECTIVE TREATMENT OF H2 OR T2. EXTERNAL TIMBER TO BE EITHER HARDWOOD DURABILITY CLASS I OR II AS PER AS1720 OR IMPREGNATED PINE, PRESSURE TREATED TO AS1604. SUPPLEMENTARY TREATMENT TO BE APPLIED TO ALL SITE CUT SURFACES.

ROOF TRUSSES DESIGNED BY THE MANUFACTURER SHALL BE TO THE RELEVANT STANDARDS. DRAWINGS TO BE SUBMITTED TO THE ENGINEER CLEARLY INDICATING THE LOADS THAT ARE IMPOSED ON THE STRUCTURE.

ALL BOLTS TO BE MINIMUM M16 4.6/S, WITH WASHER AT LEAST 2.5 x BOLT DIAMETER. ALL FASTENERS TO BE HOT DIP GALVANISED.

ALL CONNECTIONS AND BRACING TO BE CARRIED OUT IN ACCORDANCE WITH AS1684 AND AS1720.

ALL WALLS SHALL BE 90 x 45 F7 AT 450mm CENTRES UNLESS NOTED OTHERWISE. PROVIDE DOUBLE STUDS OR 90 x 90 MGP10 POSTS UNDER THE ENDS OF ALL BEAMS, LINTELS, STRUTS AND TRUSSES UNLESS NOTED OTHERWISE.

ALL LVL'S USED SHALL COMPLY WITH AS4357 AND BE INSTALLED AS PER THE MANUFACTURERS SPECIFICATIONS.

ALL JOISTS WITH A DEPTH GREATER THAN 150mm SHALL HAVE BLOCKING OVER SUPPORT BEARERS AND AT A MAXIMUM OF 3M CENTRES.



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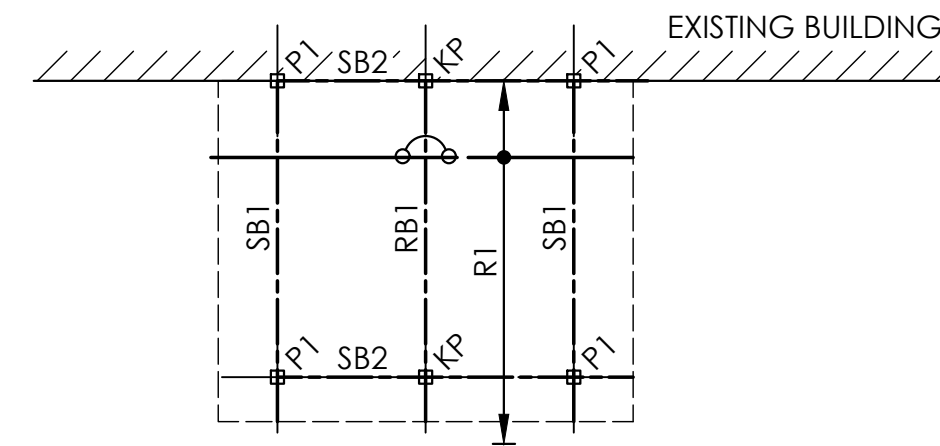
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Signed E Liang

Sheet No 12 of 20

STATUS				
PRELIMINARY				
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04	10.09.21	J.J	G.Z	
PROJECT No.		DRAWING No.		SHEET SIZE
SONNENHOFF		N2		A2



ROOF FRAMING PLAN (1:50)

----- DASHED LINES DENOTE ROOF LINE OVER.

POSTS

P1 ————— 89 x 89 x 6 SHS.
 KP ————— 75 x 75 x 5 SHS KING POST.

RB1 ————— 200 UB 25.

STEEL BEAMS
SB1 ————— 200 PFC.
SB2 ————— 200 PFC.

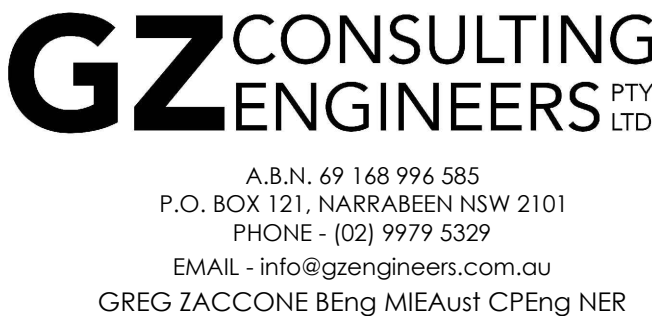
R1 ————— 12mm STEEL FLAT PLATE.

ALL BEAMS AND BRACING NOT SHOWN ON PLAN ARE
TO BE DETERMINED IN ACCORDANCE WITH AS1684
"RESIDENTIAL TIMBER FRAMING CONSTRUCTION.

XXX ——— DENOTES SLAB THICKNESS.

A ——— 450 DEEP X 450 WIDE EDGE BEAM.

TR1 ——— SL82 FABRIC, 40 TOP COVER.




PROJECT & SITE

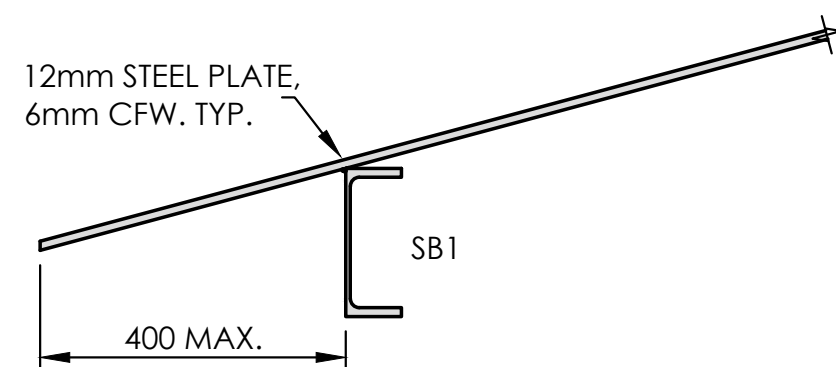
PROPOSED PORTICO AT
29 WHEATLEY ROAD,
PERISHER VALLEY, NSW 2624

TITLE

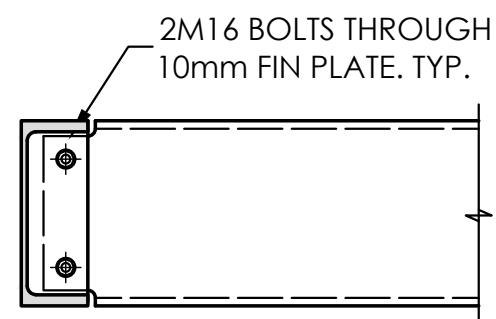
RAFT SLAB & ROOF
FRAMING PLAN

STATUS **PRELIMINARY**

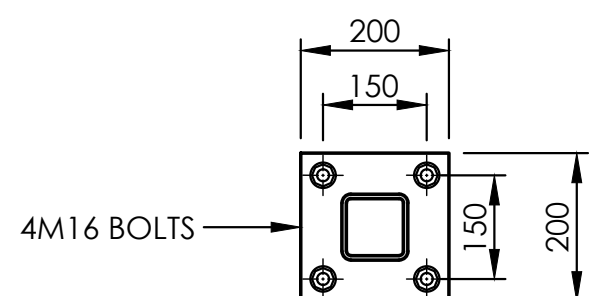
<u>SCALE</u>	<u>DATE</u>	<u>DRAWN</u>	<u>DESIGN</u>	<u>SIGNED</u>
AS SHOWN	10.09.21	J.J	G.Z	
<u>PROJECT No.</u>		<u>DRAWING No.</u>		<u>SHEET SIZE</u>
SONNENHOFF		S1		A2



STEEL PLATE TO PFC CONNECTION (1:10)

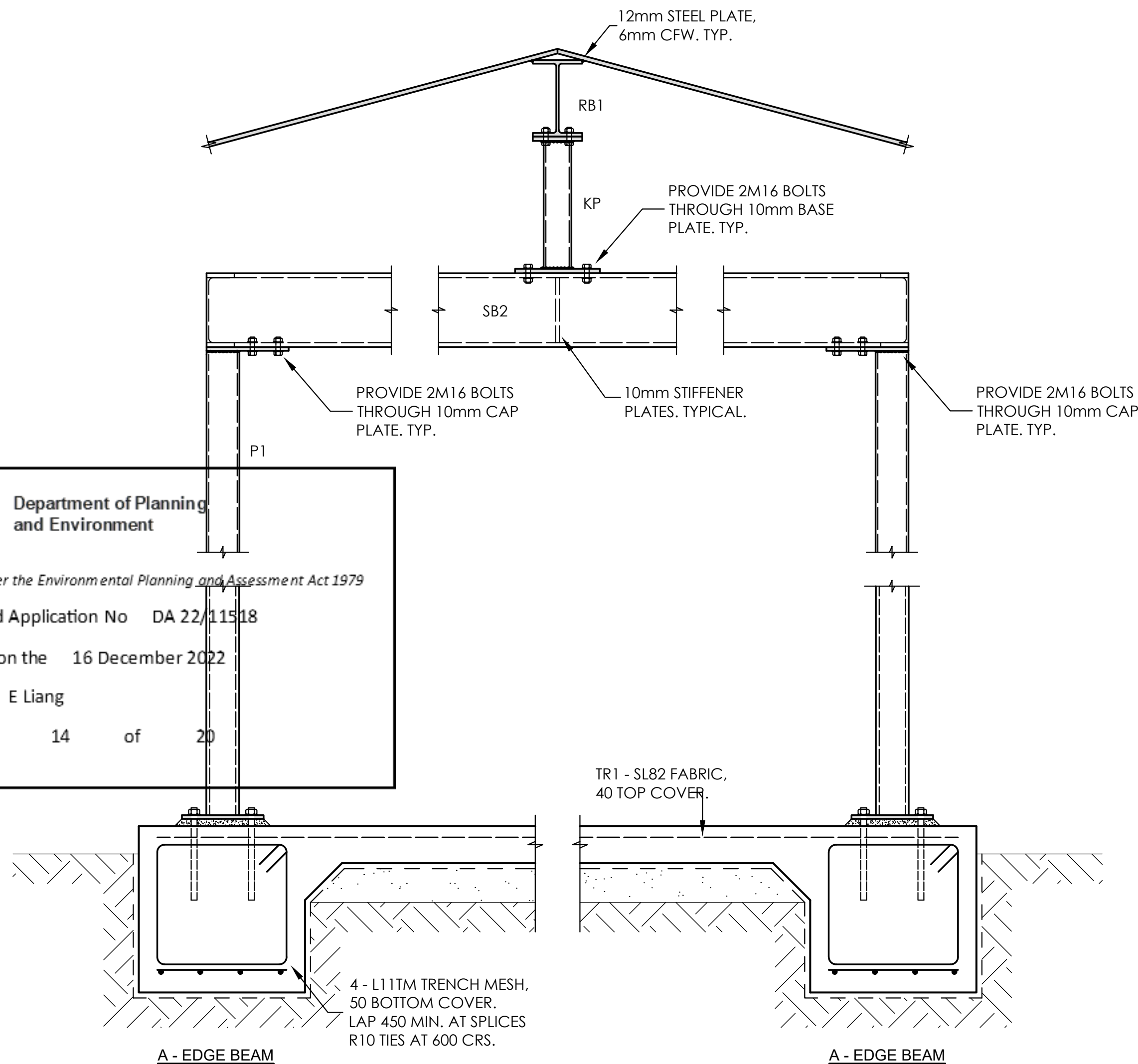
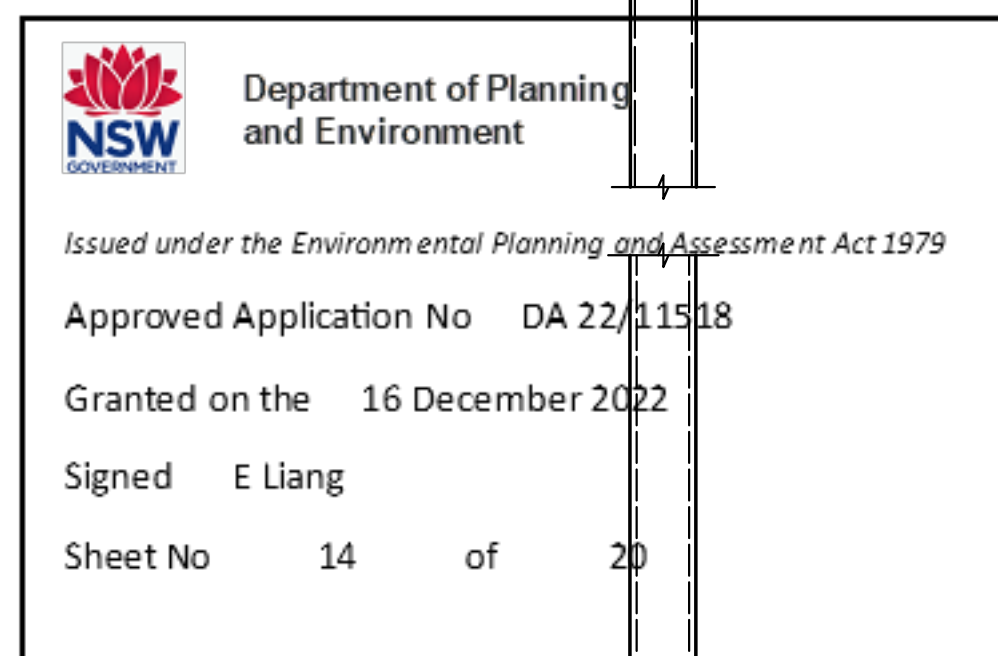


TYPICAL BEAM TO PFC CONNECTION (1:10)



BASE PLATE CONNECTIONS (1:20)

1. ALL BASE PLATES TO BE 12mm PLATE (6mm CFW)
2. ORIENTATE BASE PLATE TO SUIT WALL LOCATION
3. 4M16 CHEMSET ANCHOR BOLTS WITH 125mm EMBEDMENT



STRUCTURAL SECTION (1:10)



0	28.10.21	PRELIMINARY		<div><div><div><div>GZ</div><div>CONSULTING ENGINEERS</div></div><div>PTY LTD</div></div><div>A.B.N. 69 168 996 585 P.O. BOX 121, NARRABEEN NSW 2101 PHONE - (02) 9979 5329 EMAIL - info@gzengineers.com.au GREG ZACCONE BEng MIEAust CPEng NER</div></div>	CLIENT	CHALET SONNENHOFF	TITLE	STRUCTURAL SECTION & DETAILS	STATUS				
1	11.01.22	FOR CONSTRUCTION							PRELIMINARY				
2	29.04.22	GENERAL REVISIONS			SCALE				DATE	DRAWN	DESIGN	SIGNED	
					AS SHOWN				10.09.21	J.J	G.Z		
					PROJECT No.				DRAWING No.		SHEET SIZE		
REV	DATE	REVISIONS	SIGNED		SONNENHOFF				S2		A2		